

1-9* (10 %)

From an overall free-body diagram, the equations of equilibrium

$\rightarrow \Sigma F_x = 0:$ $A_x = 0$

$\uparrow \Sigma F_y = 0:$ $A_y - 10 - 20 + N_E = 0$

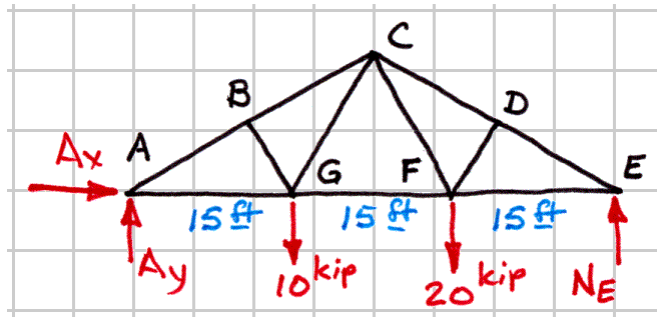
$\curvearrowright \Sigma M_A = 0:$ $45N_E - 15(10) - 30(20) = 0$

are solved to get

$A_x = 0 \text{ kip}$

$A_y = 13.3333 \text{ kip } \uparrow$

$N_E = 16.6667 \text{ kip } \uparrow$



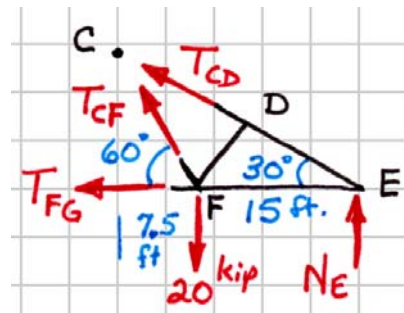
Then, from a free-body diagram of the right hand section of the truss, the equations of equilibrium

$\curvearrowright \Sigma M_F = 0:$ $15N_E + 15(T_{DE} \sin 30^\circ) = 0$

$\curvearrowright \Sigma M_E = 0:$ $15(20) - 15(T_{CF} \sin 60^\circ) = 0$

$\curvearrowright \Sigma M_C = 0:$ $22.5N_E - 7.5(20) - (15 \cos 30^\circ)T_{FG} = 0$

are solved to get



$T_{CD} = -33.3333 \text{ kip} \cong 33.3 \text{ kip (C)} \dots \text{Ans.}$

$T_{CF} = +23.094 \text{ kip} \cong 23.1 \text{ kip (T)} \dots \text{Ans.}$

$T_{FG} = +17.32 \text{ kip} = 17.32 \text{ kip (T)} \dots \text{Ans.}$

(T_{CD} 為 compressive force 要註明清楚，或以負號表示)

未標單位 扣1分/個

方向或正負號未標 扣1分/個

算式對答案錯 扣2分/個

1-17* (10 %)

$$\curvearrowright \Sigma M_A = 0: \quad 1.25 N_D - 9(25) = 0$$

$$N_D = 180.0 \text{ lb} \dots \text{Ans.}$$

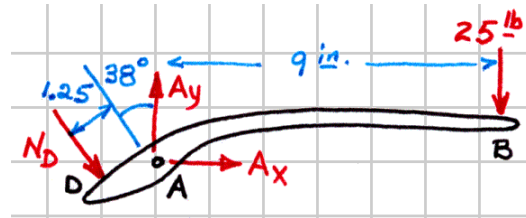
$$\rightarrow \Sigma F_x = 0: \quad A_x + N_D \sin 38^\circ = 0$$

$$\uparrow \Sigma F_y = 0: \quad A_y - 25 - N_D \cos 38^\circ = 0$$

$$A_x = -110.8 \text{ lb}$$

$$A_y = 166.8 \text{ lb}$$

$$\mathbf{A} = 200.3 \text{ lb} \searrow 56.4^\circ \dots \text{Ans.}$$



少算一個答案 扣5分/個

未標單位 扣1分/個

未標角度或方向 扣1分/個

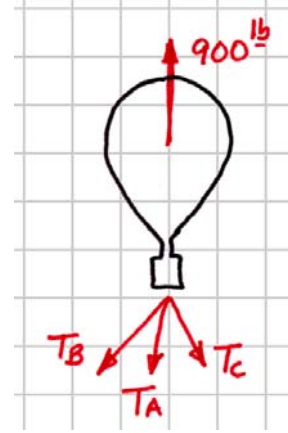
1-25* (20%)

The components of the three tension forces are

$$\begin{aligned}\mathbf{T}_A &= T_A \frac{20\mathbf{i} + 30\mathbf{j} - 50\mathbf{k}}{\sqrt{20^2 + 30^2 + 50^2}} \\ &= 0.32444T_A\mathbf{i} + 0.48666T_A\mathbf{j} - 0.81111T_A\mathbf{k}\end{aligned}$$

$$\begin{aligned}\mathbf{T}_B &= T_B \frac{16\mathbf{i} - 25\mathbf{j} - 50\mathbf{k}}{\sqrt{16^2 + 25^2 + 50^2}} \\ &= 0.27517T_B\mathbf{i} - 0.42995T_B\mathbf{j} - 0.85990T_B\mathbf{k}\end{aligned}$$

$$\begin{aligned}\mathbf{T}_C &= T_C \frac{-25\mathbf{i} - 15\mathbf{j} - 50\mathbf{k}}{\sqrt{25^2 + 15^2 + 50^2}} \\ &= -0.43193T_C\mathbf{i} - 0.25916T_C\mathbf{j} - 0.86387T_C\mathbf{k}\end{aligned}$$



Then the x -, y -, and z -components of the force equilibrium equation give

$$x: \quad 0.32444T_A + 0.27517T_B - 0.43193T_C = 0$$

$$y: \quad 0.48666T_A - 0.42995T_B - 0.25916T_C = 0$$

$$z: \quad -0.81111T_A - 0.85990T_B - 0.86387T_C + 900 = 0$$

$$T_A = 418.214 \text{ lb} \cong 418 \text{ lb} \dots\dots\dots \text{Ans.}$$

$$T_B = 205.219 \text{ lb} \cong 205 \text{ lb} \dots\dots\dots \text{Ans.}$$

$$T_C = 444.876 \text{ lb} \cong 445 \text{ lb} \dots\dots\dots \text{Ans.}$$

未標單位 扣1分/個

z 方向等式寫錯(未寫900) 扣2分/個

1-67 (20%)

From a free-body diagram of the pipe the moment equilibrium equation

$$\Sigma \mathbf{M}_{cut} = \mathbf{0} :$$

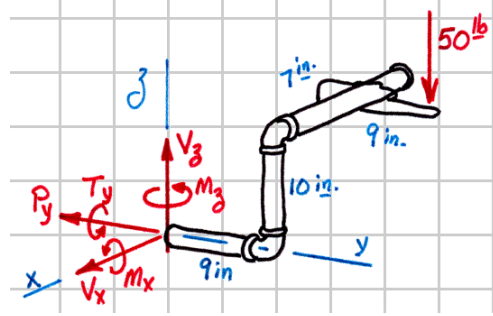
$$(M_x \mathbf{i} + T_y \mathbf{j} + M_z \mathbf{k}) + (-7\mathbf{i} + 18\mathbf{j} + 10\mathbf{k}) \times (-50\mathbf{k}) = \mathbf{0}$$

has x-, y-, and z-components

$$x : \quad M_x - 900 = 0 \quad M_x = 900 \text{ lb} \cdot \text{in.} \dots \text{Ans.}$$

$$y : \quad T_y - 350 = 0 \quad T_y = 350 \text{ lb} \cdot \text{in.} \dots \text{Ans.}$$

$$z : \quad M_z = 0 \quad M_z = 0 \text{ lb} \cdot \text{in.} \dots \text{Ans.}$$



and the force equilibrium equation has components

$$\Sigma F_x = 0 : \quad V_x = 0 \quad V_x = 0 \text{ lb} \dots \text{Ans.}$$

$$\Sigma F_y = 0 : \quad P_y = 0 \quad P_y = 0 \text{ lb} \dots \text{Ans.}$$

$$\Sigma F_z = 0 : \quad V_z - 50 = 0 \quad V_z = 50 \text{ lb} \dots \text{Ans.}$$

未標單位 扣1分/個
 少算一個答案 扣3分/個

1-77 (20%)

(a) From a free-body diagram of the entire beam, the equations of equilibrium give

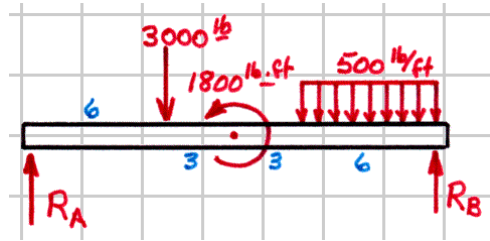
$$\uparrow \Sigma F_y = 0: \quad R_A + R_B - 3000 - 500(6) = 0$$

$$\curvearrowright \Sigma M_B = 0:$$

$$[500(6)](3) + 3000(12) + 1800 - 18R_A = 0$$

$R_A = 2600 \text{ lb}$ Ans.

$R_B = 3400 \text{ lb}$ Ans.



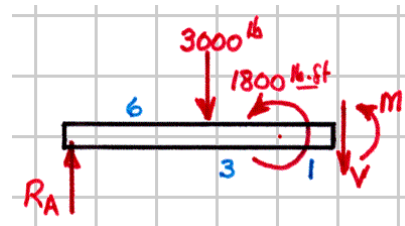
(b) Next, from a free-body diagram of the left end of the beam, the equations of equilibrium give

$$\uparrow \Sigma F_y = 0: \quad (2600) - 3000 - V = 0$$

$$\curvearrowright \Sigma M_{cut} = 0: \quad M + 1800 + 3000(4) - (2600)(10) = 0$$

$V = -400 \text{ lb}$ Ans.

$M = 12,200 \text{ lb} \cdot \text{ft}$ Ans.



未標單位 扣1分/個

少算一個答案 扣5分/個

若圖上標示V為向上則答案V會為 +400 lb (答案需與所標示方向相符)

1-83 (20%)

From a free-body diagram of the upper half of the clamp, the equations of equilibrium give

$$\rightarrow \Sigma F_x = 0: \quad V = 0$$

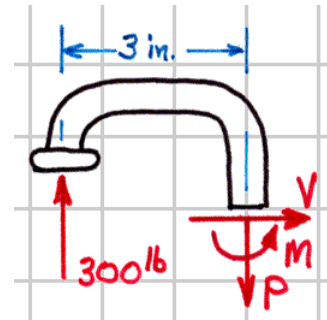
$$\uparrow \Sigma F_y = 0: \quad 300 - P = 0$$

$$\curvearrowright \Sigma M_{cut} = 0: \quad M - 3(300) = 0$$

$$P = 300 \text{ lb (T)} \dots\dots\dots \text{Ans.}$$

$$V = 0 \text{ lb} \dots\dots\dots \text{Ans.}$$

$$M = 900 \text{ lb} \cdot \text{ft} \dots\dots\dots \text{Ans.}$$



未標單位 扣1分/個

少算一個答案 扣6分/個

(單位可能隨版本不同，但請在圖上標上之後答案的單位也要一致)

($V=0$ 也要寫出來)